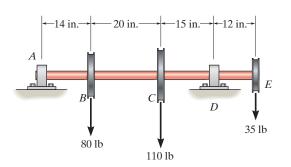
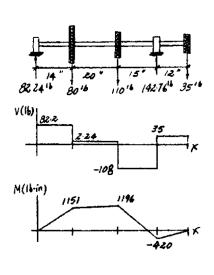
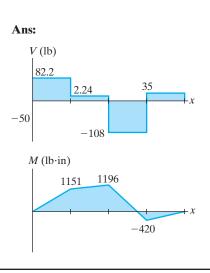
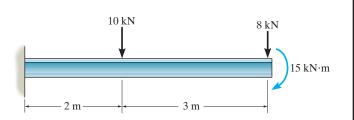
6–2. Draw the shear and moment diagrams for the shaft. The bearings at A and D exert only vertical reaction on the shaft. The loading is applied to the pulleys at B and C and E.

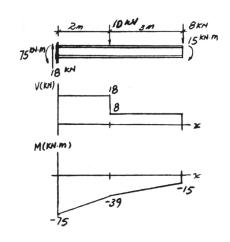


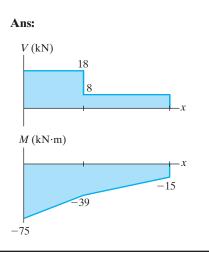




6–5. Draw the shear and moment diagrams for the beam.







6–18. Draw the shear and moment diagrams for the beam, and determine the shear and moment throughout the beam as functions of x.

Support Reactions: As shown on FBD.

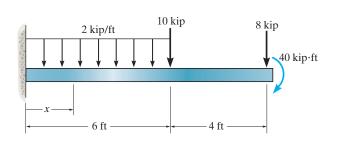
Shear and Moment Function:

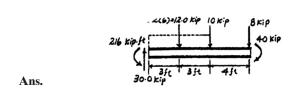
For $0 \le x < 6$ ft:

$$+\uparrow \Sigma F_y = 0;$$
 $30.0 - 2x - V = 0$ $V = \{30.0 - 2x\} \text{ kip}$ $\zeta + \Sigma M_{NA} = 0;$ $M + 216 + 2x \left(\frac{x}{2}\right) - 30.0x = 0$ $M = \{-x^2 + 30.0x - 216\} \text{ kip} \cdot \text{ft}$

For 6 ft $< x \le 10$ ft:

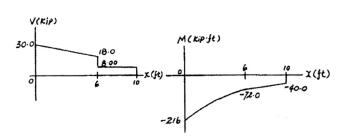
$$+\uparrow \Sigma F_y = 0;$$
 $V - 8 = 0$ $V = 8.00 \text{ kip}$ $\zeta + \Sigma M_{NA} = 0;$ $-M - 8(10 - x) - 40 = 0$ $M = \{8.00x - 120\} \text{ kip} \cdot \text{ft}$





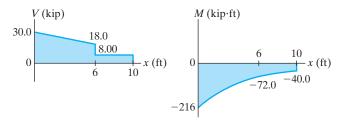




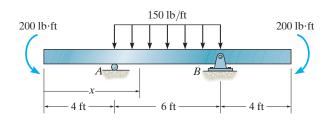


Ans:

For
$$0 \le x < 6$$
 ft: $V = \{30.0 - 2x\}$ kip,
 $M = \{-x^2 + 30.0x - 216\}$ kip · ft,
For 6 ft $< x \le 10$ ft: $V = 8.00$ kip,
 $M = \{8.00x - 120\}$ kip · ft



6–25. Draw the shear and moment diagrams for the beam and determine the shear and moment in the beam as functions of x, where 4 ft < x < 10 ft.



$$+\uparrow \Sigma F_v = 0;$$
 $-150(x - 4) - V + 450 = 0$

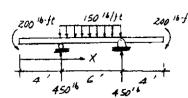
$$V = 1050 - 150x$$

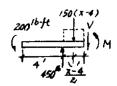
Ans.

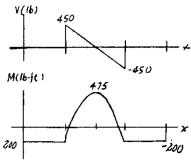
$$\zeta + \Sigma M = 0;$$
 $-200 - 150(x - 4)\frac{(x - 4)}{2} - M + 450(x - 4) = 0$

$$M = -75x^2 + 1050x - 3200$$

Ans.

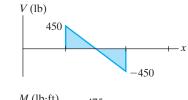


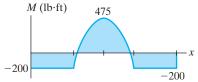




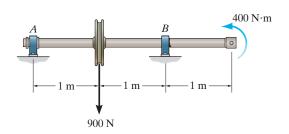
Ans:

$$V = 1050 - 150x$$
$$M = -75x^2 + 1050x - 3200$$





6–33. The shaft is supported by a smooth thrust bearing at A and smooth journal bearing at B. Draw the shear and moment diagrams for the shaft.



Equations of Equilibrium: Referring to the free-body diagram of the shaft shown in Fig. a,

$$\zeta + \Sigma M_A = 0;$$

$$\zeta + \Sigma M_A = 0;$$
 $B_y(2) + 400 - 900(1) = 0$

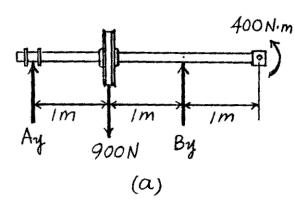
$$B_{y} = 250 \text{ N}$$

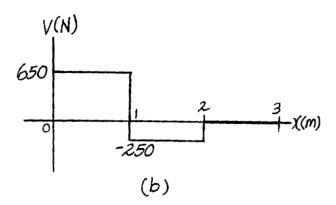
$$+\uparrow \Sigma F_{\nu} = 0$$
:

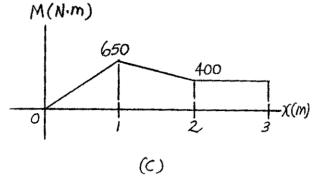
$$+\uparrow \Sigma F_y = 0;$$
 $A_y + 250 - 900 = 0$

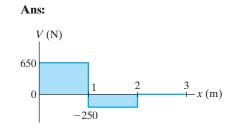
$$A_{v} = 650 \,\mathrm{N}$$

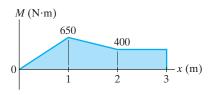
Shear and Moment Diagram: As shown in Figs. b and c.



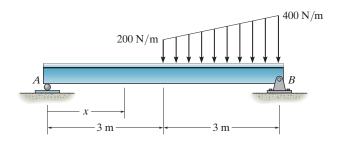








6-35. Draw the shear and moment diagrams for the beam and determine the shear and moment as functions of x.



Support Reactions: As shown on FBD.

Shear and Moment Functions:

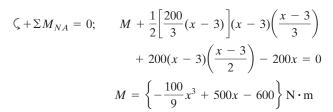
For $0 \le x < 3$ m:

$$+\uparrow \Sigma F_y = 0;$$
 $200 - V = 0$ $V = 200 \text{ N}$ $\zeta + \Sigma M_{NA} = 0;$ $M - 200 x = 0$ $M = \{200 x\} \text{ N} \cdot \text{m}$

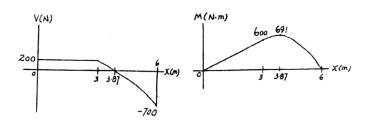
For $3 \text{ m} < x \le 6 \text{ m}$:

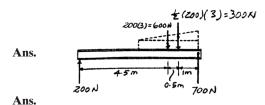
$$+\uparrow \Sigma F_y = 0;$$
 $200 - 200(x - 3) - \frac{1}{2} \left[\frac{200}{3} (x - 3) \right] (x - 3) - V = 0$
$$V = \left\{ -\frac{100}{3} x^2 + 500 \right\} N$$

Set V = 0, x = 3.873 m



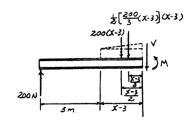
Substitute $x = 3.87 \text{ m}, M = 691 \text{ N} \cdot \text{m}$





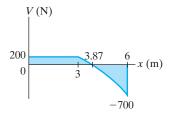


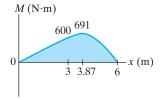
Ans.



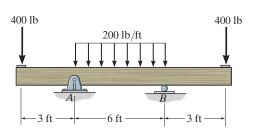
Ans.

For
$$0 \le x < 3$$
 m: $V = 200$ N, $M = (200x)$ N·m,
For 3 m $< x \le 6$ m: $V = \left\{-\frac{100}{3}x^2 + 500\right\}$ N,
 $M = \left\{-\frac{100}{9}x^3 + 500x - 600\right\}$ N·m





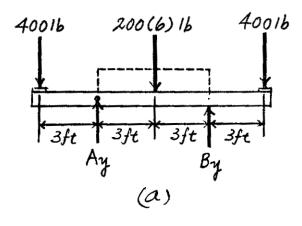
6–39. Draw the shear and moment diagrams for the double overhanging beam.

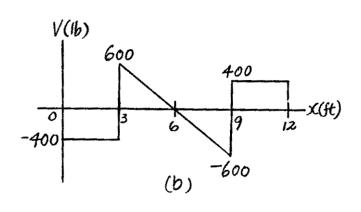


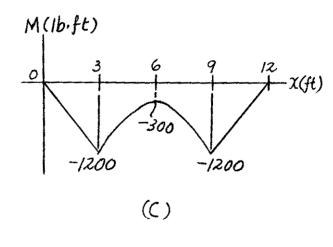
Equations of Equilibrium: Referring to the free-body diagram of the beam shown in Fig. a,

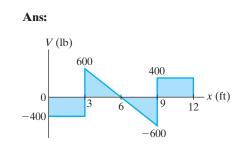
$$\zeta + \Sigma M_A = 0;$$
 $B_y(6) + 400(3) - 200(6)(3) - 400(9) = 0$ $B_y = 1000 \text{ lb}$ $A_y + 1000 - 400 - 200(6) - 400 = 0$ $A_y = 1000 \text{ lb}$

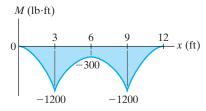
Shear and Moment Diagram: As shown in Figs. b and c.





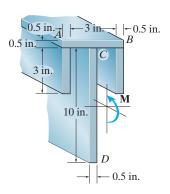






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6–49. Determine the maximum tensile and compressive bending stress in the beam if it is subjected to a moment of $M = 4 \text{ kip} \cdot \text{ft}$.



Section Properties:

$$\overline{y} = \frac{\sum \widetilde{y}A}{\sum A}$$

$$= \frac{0.25(4)(0.5) + 2[2(3)(0.5)] + 5.5(10)(0.5)}{4(0.5) + 2[(3)(0.5)] + 10(0.5)} = 3.40 \text{ in.}$$

$$I_{NA} = \frac{1}{12}(4)(0.5^3) + 4(0.5)(3.40 - 0.25)^2 + 2\left[\frac{1}{12}(0.5)(3^3) + 0.5(3)(3.40 - 2)^2\right] + \frac{1}{12}(0.5)(10^3) + 0.5(10)(5.5 - 3.40)^2$$

$$= 91.73 \text{ in}^4$$

Maximum Bending Stress: Applying the flexure formula $\sigma_{\max} = \frac{Mc}{I}$

$$(\sigma_t)_{\text{max}} = \frac{4(10^3)(12)(10.5 - 3.40)}{91.73} = 3715.12 \text{ psi} = 3.72 \text{ ksi}$$

$$4(10^3)(12)(3.40)$$
Ans.

$$(\sigma_c)_{\text{max}} = \frac{4(10^3)(12)(3.40)}{91.73} = 1779.07 \text{ psi} = 1.78 \text{ ksi}$$
 Ans.



Ans:

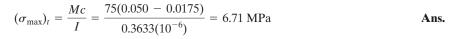
$$(\sigma_t)_{\text{max}} = 3.72 \text{ ksi}, (\sigma_c)_{\text{max}} = 1.78 \text{ ksi}$$

6–59. The aluminum machine part is subjected to a moment of $M = 75 \text{ kN} \cdot \text{m}$. Determine the maximum tensile and compressive bending stresses in the part.

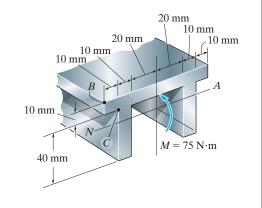
$$\overline{y} = \frac{0.005(0.08)(0.01) + 2[0.03(0.04)(0.01)]}{0.08(0.01) + 2(0.04)(0.01)} = 0.0175 \text{ m}$$

$$I = \frac{1}{12}(0.08)(0.01^3) + 0.08(0.01)(0.0125^2)$$

$$+ 2\left[\frac{1}{12}(0.01)(0.04^3) + 0.01(0.04)(0.0125^2)\right] = 0.3633(10^{-5}) \text{ m}^4$$



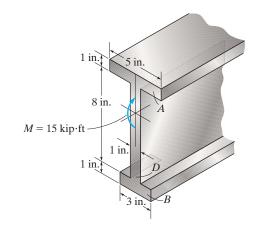
$$(\sigma_{\text{max}})_c = \frac{My}{I} = \frac{75(0.0175)}{0.3633(10^{-6})} = 3.61 \text{ MPa}$$



Ans:

 $(\sigma_{\text{max}})_t = 6.71 \text{ MPa}, (\sigma_{\text{max}})_c = 3.61 \text{ MPa}$

6–61. The beam is subjected to a moment of $15 \text{ kip} \cdot \text{ft}$. Determine the percentage of this moment that is resisted by the web D of the beam.



$$\overline{y} = \frac{\Sigma \widetilde{y} A}{\Sigma A} = \frac{0.5(1)(5) + 5(8)(1) + 9.5(3)(1)}{1(5) + 8(1) + 3(1)} = 4.4375 \text{ in.}$$

$$I = \frac{1}{12}(5)(1^3) + 5(1)(4.4375 - 0.5)^2 + \frac{1}{12}(1)(8^3) + 8(1)(5 - 4.4375)^2 + \frac{1}{12}(3)(1^3) + 3(1)(9.5 - 4.4375)^2$$

 $= 200.27 \text{ in}^4$

Using flexure formula $\sigma = \frac{My}{I}$

$$\sigma_A = \frac{15(12)(4.4375 - 1)}{200.27} = 3.0896 \text{ ksi}$$

$$\sigma_B = \frac{15(12)(9-4.4375)}{200.27} = 4.1007 \text{ ksi}$$

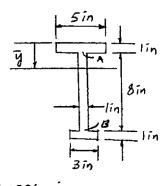
$$F_C = \frac{1}{2}(3.0896)(3.4375)(1) = 5.3102 \text{ kip}$$

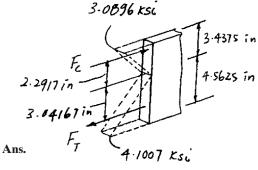
$$F_T = \frac{1}{2}(4.1007)(4.5625)(1) = 9.3547 \text{ kip}$$

$$M = 5.3102(2.2917) + 9.3547(3.0417)$$

= 40.623 kip·in. = 3.3852 kip·ft

% of moment carried by web = $\frac{3.3852}{15} \times 100 = 22.6$ %

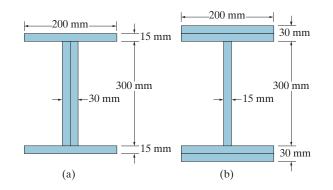




Ans:

% of moment carried by web = 22.6 %

6–69. Two designs for a beam are to be considered. Determine which one will support a moment of $M = 150 \text{ kN} \cdot \text{m}$ with the least amount of bending stress. What is that stress?



Section Property:

For section (a)

$$I = \frac{1}{12}(0.2)(0.33^3) - \frac{1}{12}(0.17)(0.3)^3 = 0.21645(10^{-3}) \text{ m}^4$$

For section (b)

$$I = \frac{1}{12}(0.2)(0.36^3) - \frac{1}{12}(0.185)(0.3^3) = 0.36135(10^{-3}) \,\mathrm{m}^4$$

Maximum Bending Stress: Applying the flexure formula $\sigma_{\rm max} = \frac{Mc}{I}$

For section (a)

$$\sigma_{\text{max}} = \frac{150(10^3)(0.165)}{0.21645(10^{-3})} = 114.3 \text{ MPa}$$

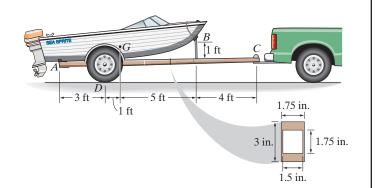
For section (b)

$$\sigma_{\min} = \frac{150(10^3)(0.18)}{0.36135(10^{-3})} = 74.72 \text{ MPa} = 74.7 \text{ MPa}$$

Ans.

Ans: $\sigma_{\min} = 74.7 \text{ MPa}$

6–71. The boat has a weight of 2300 lb and a center of gravity at G. If it rests on the trailer at the smooth contact A and can be considered pinned at B, determine the absolute maximum bending stress developed in the main strut of the trailer. Consider the strut to be a box-beam having the dimensions shown and pinned at C.



Boat:

$$+\uparrow \Sigma F_y = 0;$$
 1277.78 - 2300 + $B_y = 0$
 $B_y = 1022.22 \text{ lb}$

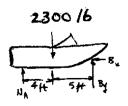
Assembly:

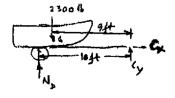
$$\zeta + \Sigma M_C = 0;$$
 $-N_D(10) + 2300(9) = 0$ $N_D = 2070 \text{ lb}$

$$+\uparrow \Sigma F_y = 0;$$
 $C_y + 2070 - 2300 = 0$ $C_y = 230 \text{ lb}$

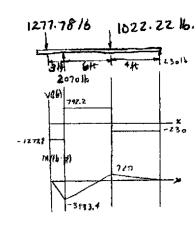
$$I = \frac{1}{12}(1.75)(3)^3 - \frac{1}{12}(1.5)(1.75)^3 = 3.2676 \text{ in}^4$$

$$\sigma_{\text{max}} = \frac{Mc}{I} = \frac{3833.3(12)(1.5)}{3.2676} = 21.1 \text{ ksi}$$





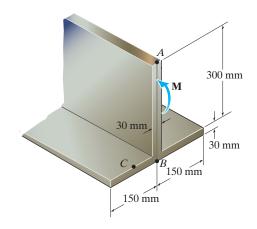
Ans.



Ans:

 $\sigma_{\rm max} = 21.1 \, \rm ksi$

6–81. If the beam is made of material having an allowable tensile and compressive stress of $(\sigma_{\rm allow})_t = 125$ MPa and $(\sigma_{\rm allow})_c = 150$ MPa, respectively, determine the maximum allowable internal moment **M** that can be applied to the beam



Section Properties: The neutral axis passes through centroid C of the cross section as shown in Fig. a. The location of C is

$$\overline{y} = \frac{\sum \widetilde{y}A}{\sum A} = \frac{0.015(0.03)(0.3) + 0.18(0.3)(0.03)}{0.03(0.3) + 0.3(0.03)} = 0.0975 \text{ m}$$

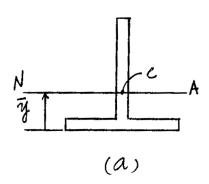
Thus, the moment of inertia of the cross section about the neutral axis is

$$I = \frac{1}{12}(0.3)(0.03^3) + 0.3(0.03)(0.0975 - 0.015)^2 + \frac{1}{12}(0.03)(0.3^3)$$
$$+ 0.03(0.3)(0.18 - 0.0975)^2$$
$$= 0.1907(10^{-3}) \text{ m}^4$$

Allowable Bending Stress: The maximum compressive and tensile stress occurs at the top and bottom-most fibers of the cross section. For the top-most fiber,

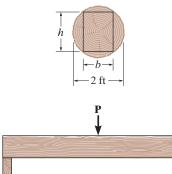
$$(\sigma_{\text{allow}})_c = \frac{Mc}{I}$$
 $150(10^6) = \frac{M(0.33 - 0.0975)}{0.1907(10^{-3})}$ $M = 123024.19 \text{ N} \cdot \text{m} = 123 \text{ kN} \cdot \text{m (controls)}$ Ans.

For the bottom-most fiber,



Ans: $M = 123 \text{ kN} \cdot \text{m}$

6–97. A log that is 2 ft in diameter is to be cut into a rectangular section for use as a simply supported beam. If the allowable bending stress for the wood is $\sigma_{\rm allow}=8$ ksi, determine the largest load P that can be supported if the width of the beam is b=8 in.



$$24^2 = h^2 + 8^2$$

$$h = 22.63 \text{ in.}$$

$$M_{\text{max}} = \frac{P}{2}(96) = 48 P$$

$$\sigma_{
m allow} = rac{M_{
m max}c}{I}$$

$$8(10^3) = \frac{48P(\frac{22.63}{2})}{\frac{1}{12}(8)(22.63)^3}$$

$$P = 114 \text{ kip}$$



Ans.



Ans: P = 114 kip